

Nonlinear Multi-Scale Finite Element Modeling of the Progressive Collapse of Reinforced Concrete Structures

Lecture 5: Corotational Bernoulli beam

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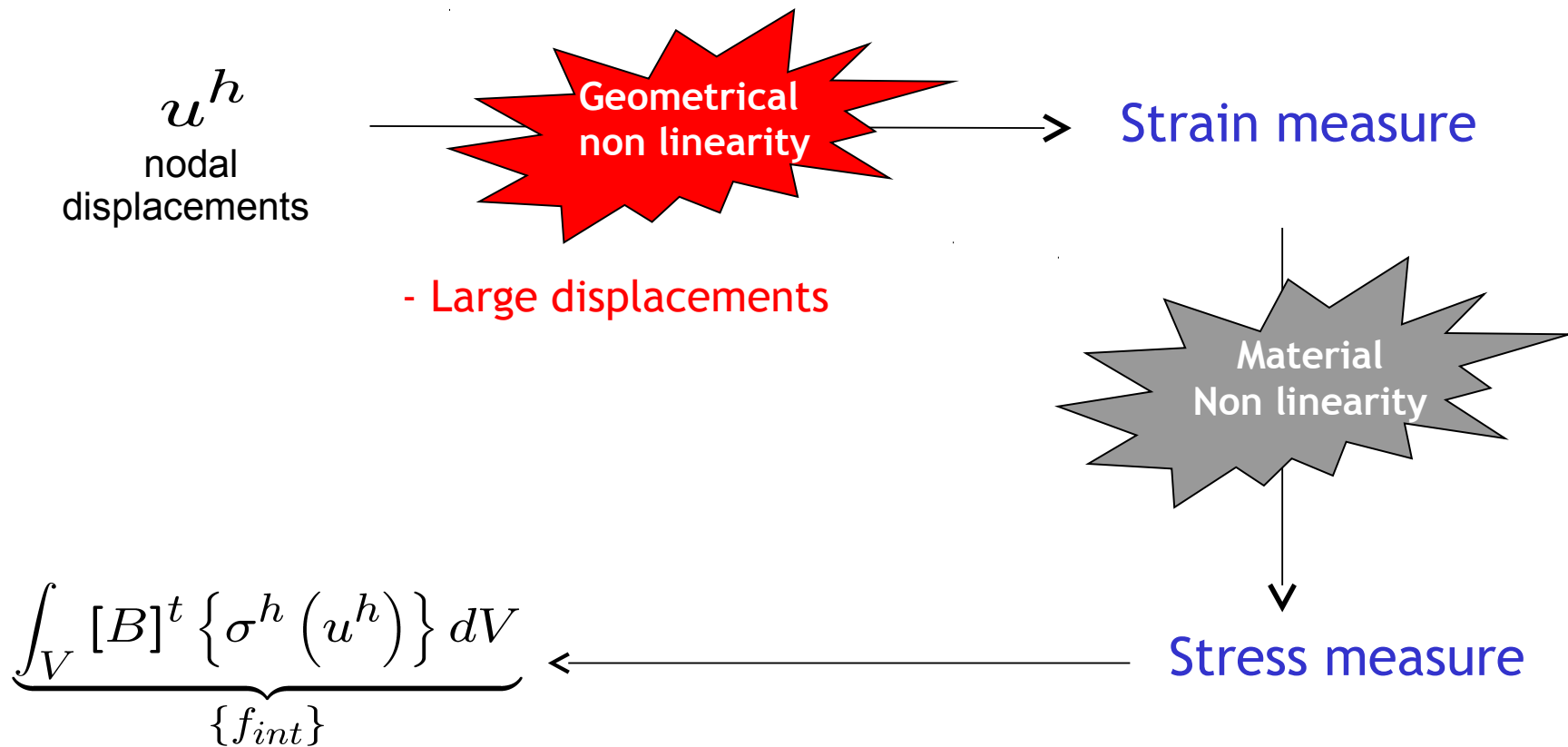
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Non proportionnality between applied forces and displacements



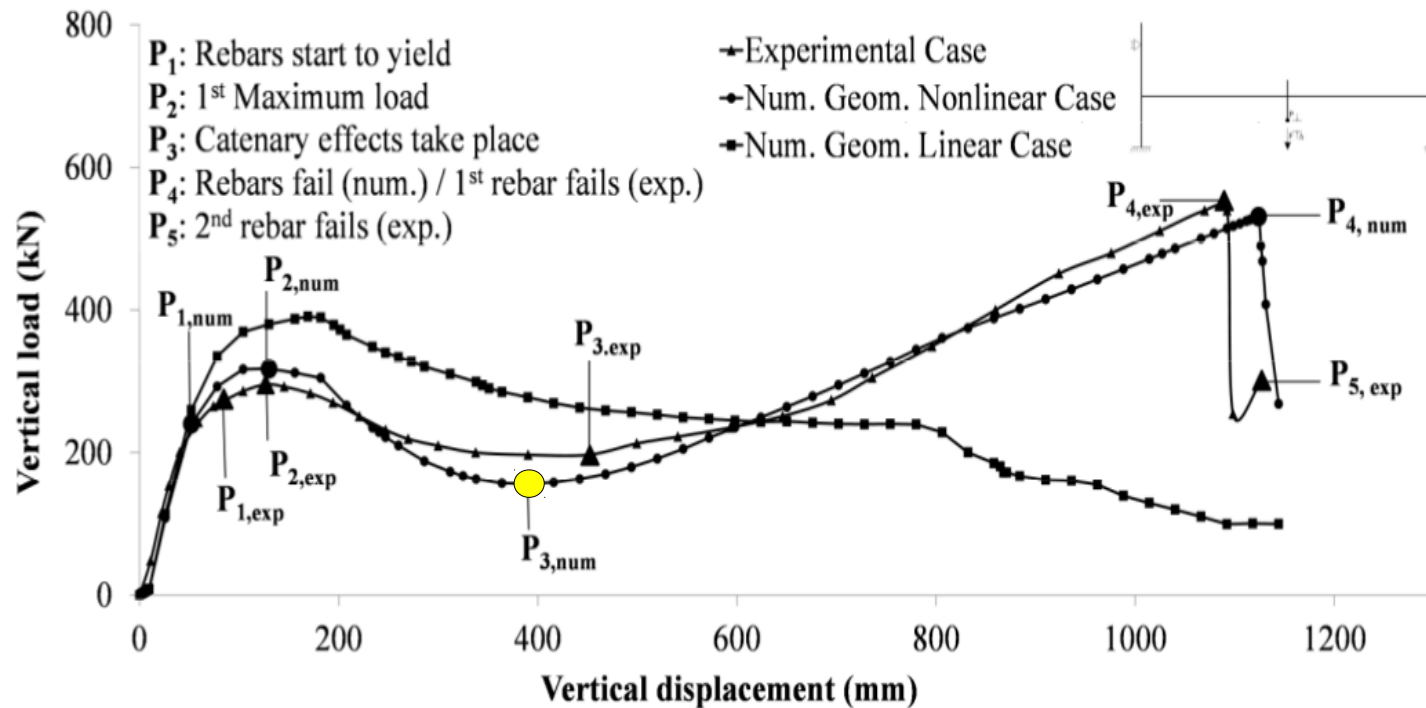
Geometrically NL formulation in PC? ³



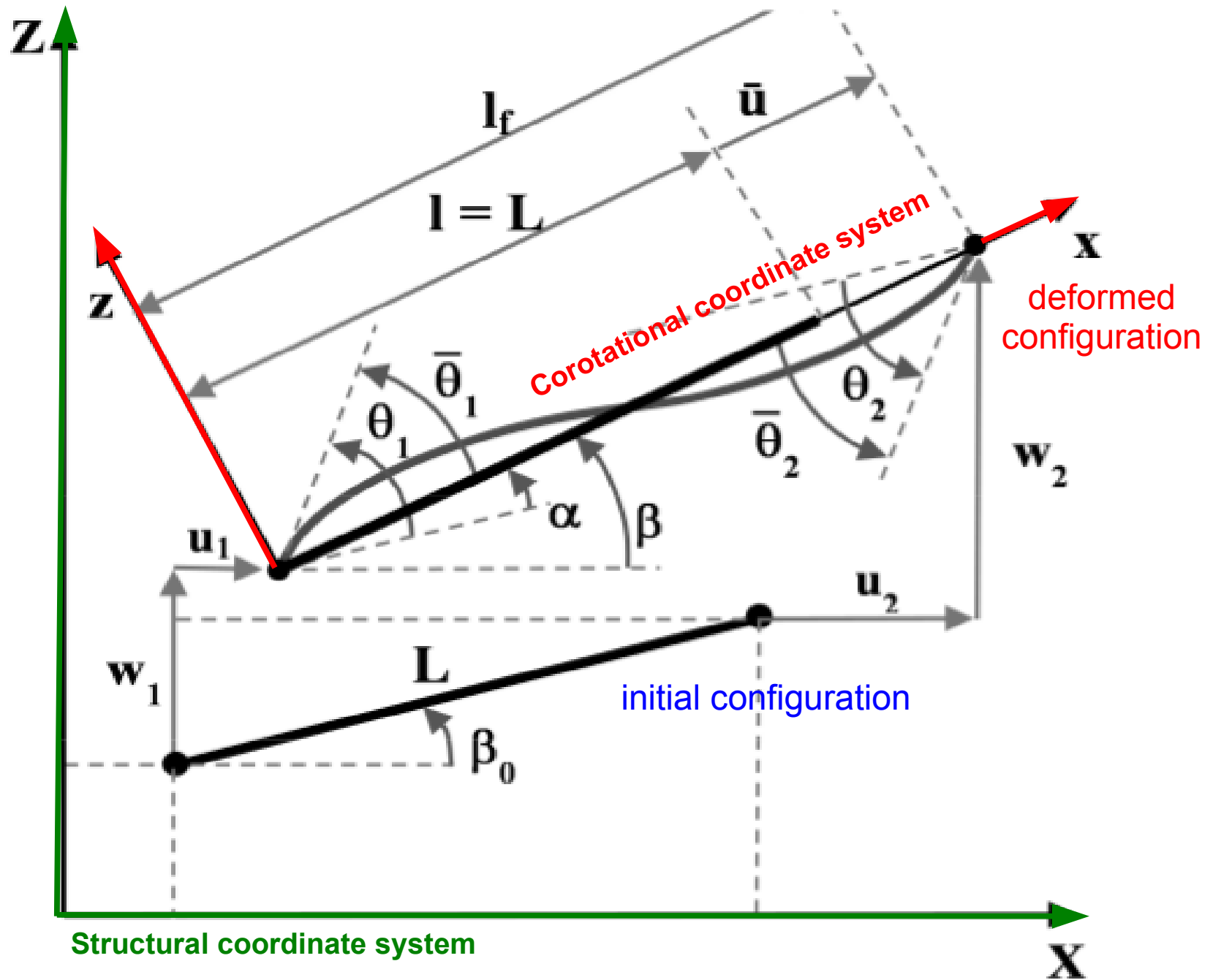
Catenary effect

- Possible strength enhancement
- Changes in the shape have to be taken into account
- Finite displacement formulation

Example of nonlinear structural response 4



Corotational Bernoulli beam formulation 5





$$\mathbf{q}_{global}^T = \left\{ u_1 \quad w_1 \quad \theta_1 \quad u_2 \quad w_2 \quad \theta_2 \right\}$$

$$\begin{bmatrix} \bar{u} \\ \bar{\theta}_1 \\ \bar{\theta}_2 \end{bmatrix} = \begin{bmatrix} l_f - L \\ \theta_1 - \alpha \\ \theta_2 - \alpha \end{bmatrix} = \begin{bmatrix} l_f - l \\ \theta_1 - \beta - \beta_0 \\ \theta_2 - \beta - \beta_0 \end{bmatrix}$$

rigid rotation

$$\mathbf{q}_{local}^T = \left\{ \bar{u} \quad \bar{\theta}_1 \quad \bar{\theta}_2 \right\}$$

$$c_o = \cos \beta_o = \frac{1}{L}(X_2 - X_1)$$

$$s_o = \sin \beta_o = \frac{1}{L}(Z_2 - Z_1)$$

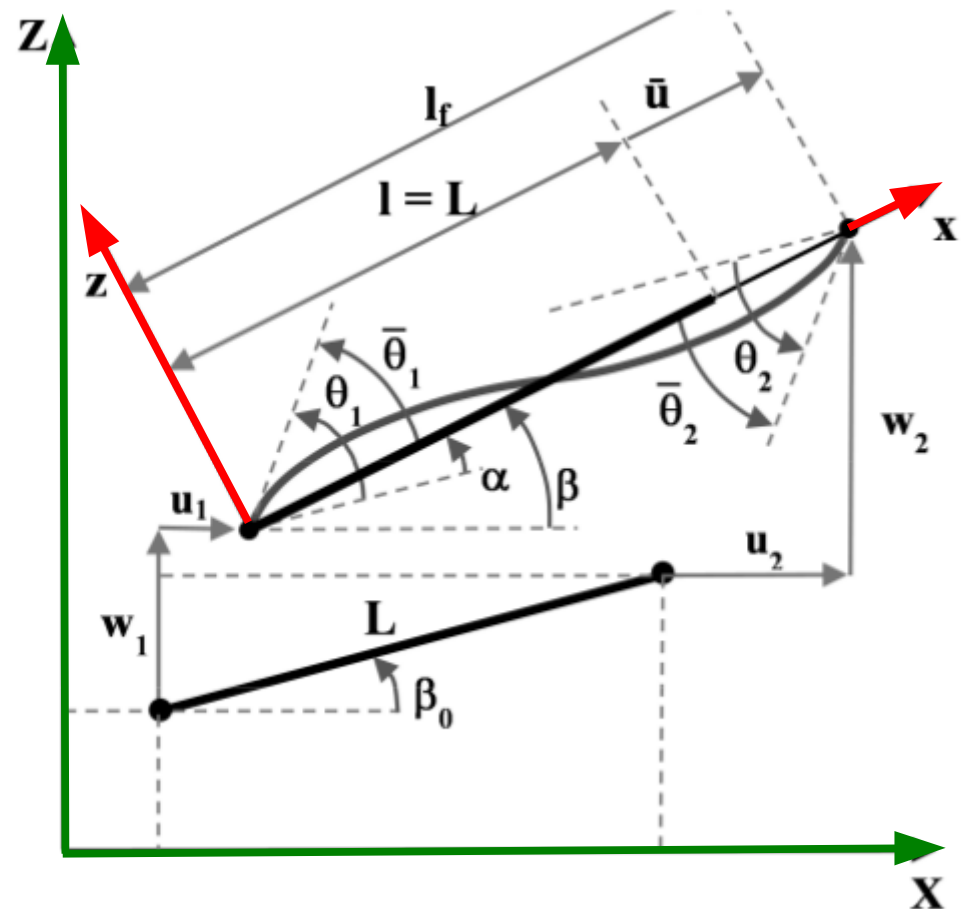
$$c = \cos \beta = \frac{1}{l_f}(X_2 - X_1 + u_2 - u_1)$$

$$s = \sin \beta = \frac{1}{l_f}(Z_2 - Z_1 + w_2 - w_1)$$

$$\sin \alpha = c_o s - s_o c$$

$$\cos \alpha = c_o c - s_o s$$

rigid rotation





Strains and stresses in corotational system 7

$$\mathbf{q}_{local}^T = \left\{ \bar{u} \quad \bar{\theta}_1 \quad \bar{\theta}_2 \right\}$$

naturally decoupled from rigid rotation

Interpolation of local displacements

$$u_{local} = \frac{x}{l} \bar{u}$$

$$w_{local} = x \left(1 - \frac{x}{l} \right)^2 \bar{\theta}_1 + \frac{x^2}{l} \left(\frac{x}{l} - 1 \right) \bar{\theta}_2$$

Generalized strains

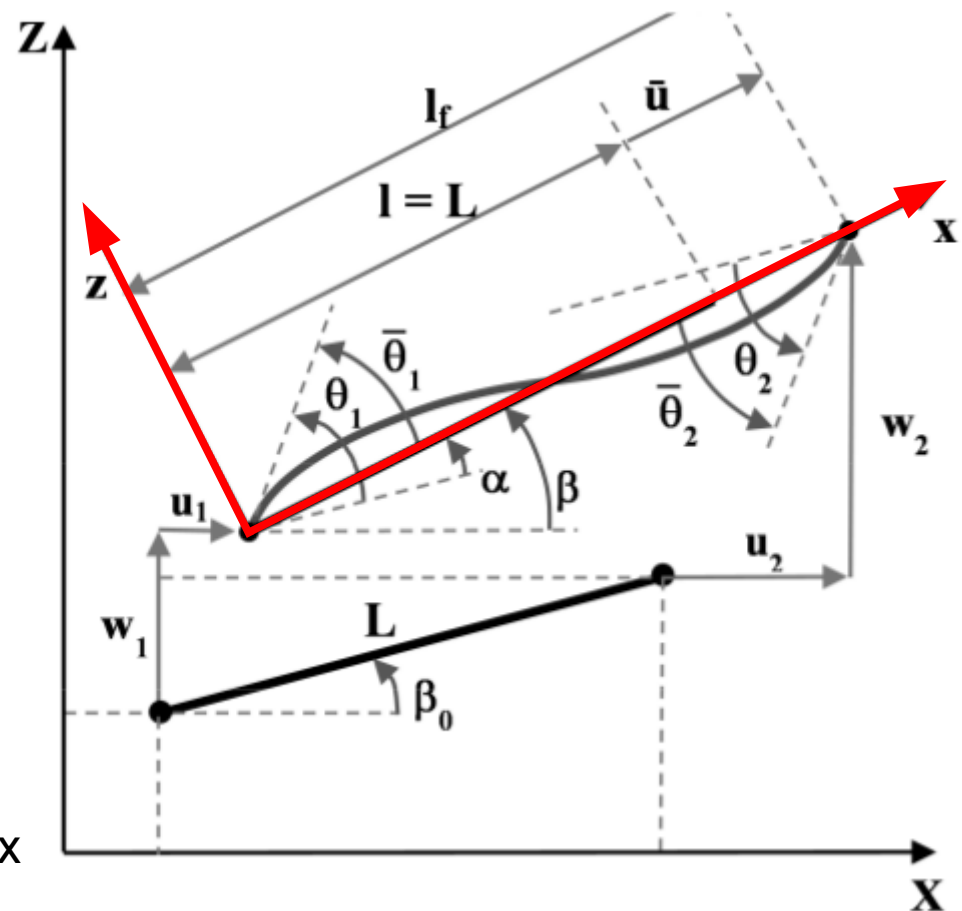
$$\begin{bmatrix} \bar{\epsilon} \\ \bar{\chi} \end{bmatrix} = \begin{bmatrix} \frac{\partial u_{local}}{\partial x} \\ \frac{\partial^2 w_{local}}{\partial x^2} \end{bmatrix}$$

Generalized stresses

$$N = \int_{a_{cor}} \sigma da$$

$$M = - \int_{a_{cor}} \sigma z da$$

vary
along x





Strains and stresses in corotational system 8

$$\mathbf{q}_{local}^T = \left\{ \bar{u} \quad \bar{\theta}_1 \quad \bar{\theta}_2 \right\}$$

naturally decoupled from rigid rotation

Interpolation of local displacements

$$u_{local} = \frac{x}{l} \bar{u}$$

$$w_{local} = x \left(1 - \frac{x}{l} \right)^2 \bar{\theta}_1 + \frac{x^2}{l} \left(\frac{x}{l} - 1 \right) \bar{\theta}_2$$

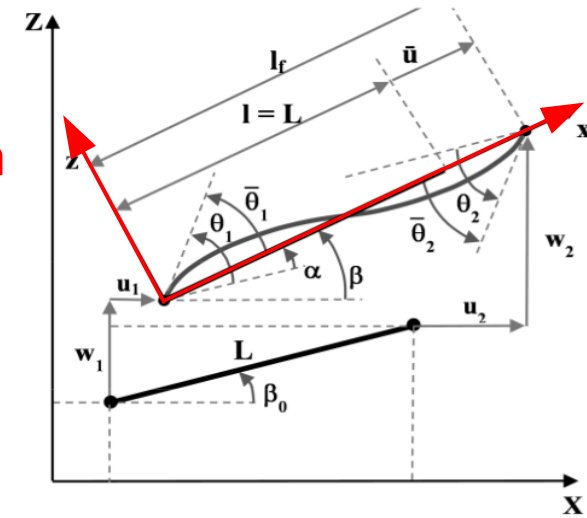
Axial strain at a given beam depth (Bernoulli)

$$\varepsilon(x, z) = \frac{\partial u_{local}}{\partial x} - \chi z = \frac{\bar{u}}{l} - z \left[\left(6 \frac{x}{l^2} - \frac{4}{l} \right) \bar{\theta}_1 + \left(6 \frac{x}{l^2} - \frac{2}{l} \right) \bar{\theta}_2 \right]$$

Internal work

$$W_{int} = \int_{v_{cor}} \sigma \delta \varepsilon dv$$

$$W_{int} = \int_{v_{cor}} \sigma \left\{ \frac{\delta \bar{u}}{l} - z \left[\left(6 \frac{x}{l^2} - \frac{4}{l} \right) \delta \bar{\theta}_1 + \left(6 \frac{x}{l^2} - \frac{2}{l} \right) \delta \bar{\theta}_2 \right] \right\} dv$$



Strains and stresses in corotational system 9

$$\mathbf{q}_{local}^T = \left\{ \bar{u} \quad \bar{\theta}_1 \quad \bar{\theta}_2 \right\}$$

naturally decoupled from
rigid rotation

Internal work

$$W_{int} = \int_{v_{cor}} \sigma \delta \varepsilon dv$$

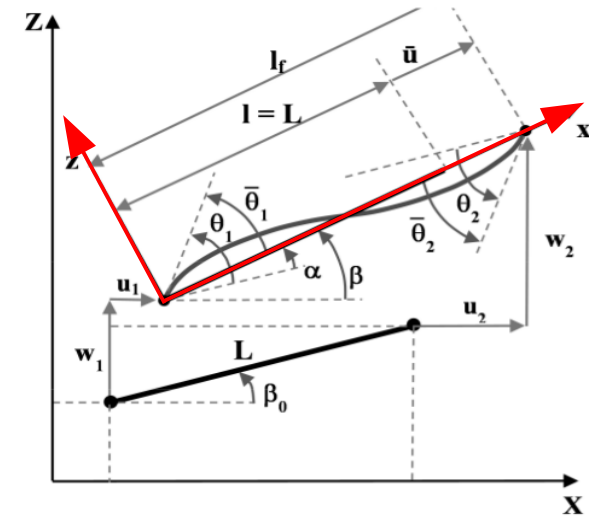
$$W_{int} = \int_{v_{cor}} \sigma \left\{ \frac{\delta \bar{u}}{l} - z \left[\left(6 \frac{x}{l^2} - \frac{4}{l} \right) \delta \bar{\theta}_1 + \left(6 \frac{x}{l^2} - \frac{2}{l} \right) \delta \bar{\theta}_2 \right] \right\} dv$$

Internal forces

$$\bar{N} = \int_{v_{cor}} \frac{\sigma}{l} dv = \int_l \frac{N}{l} dl$$

$$\bar{M}_1 = - \int_{v_{cor}} \sigma z \left(6 \frac{x}{l^2} - \frac{4}{l} \right) dv = \int_l M \left(6 \frac{x}{l^2} - \frac{4}{l} \right) dl$$

$$\bar{M}_2 = - \int_{v_{cor}} \sigma z \left(6 \frac{x}{l^2} - \frac{2}{l} \right) dv = \int_l M \left(6 \frac{x}{l^2} - \frac{2}{l} \right) dl$$



with

$$\begin{cases} N = \int_{a_{cor}} \sigma da \\ M = - \int_{a_{cor}} \sigma z da \end{cases}$$

evaluated at
Gauss points

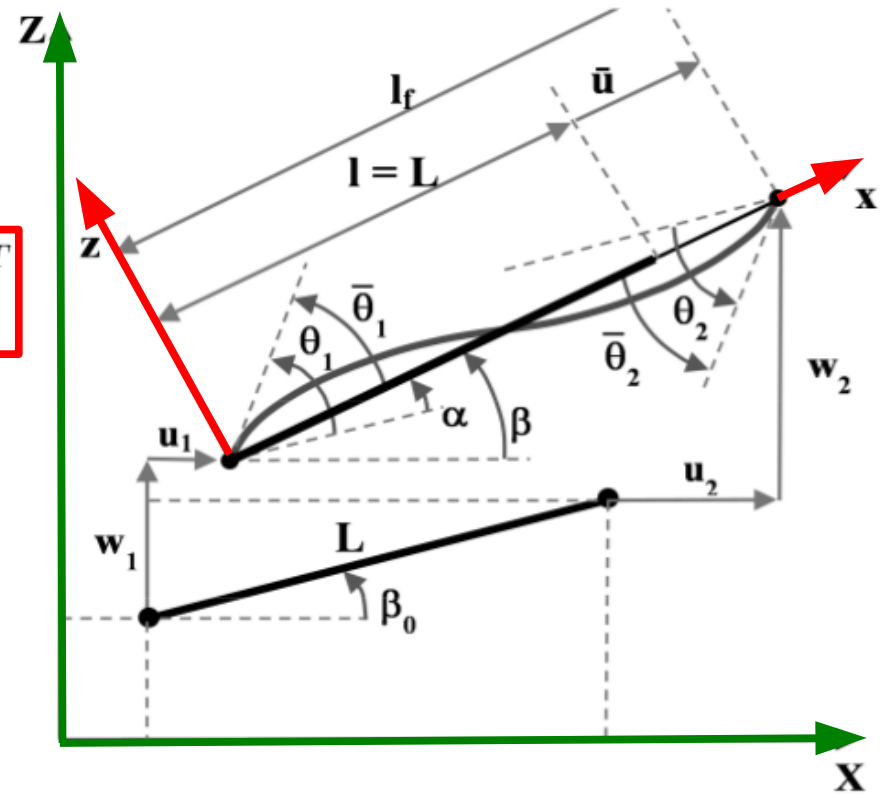
conjugated

$$\mathbf{q}_{global}^T = \left\{ u_1 \quad w_1 \quad \theta_1 \quad u_2 \quad w_2 \quad \theta_2 \right\}$$

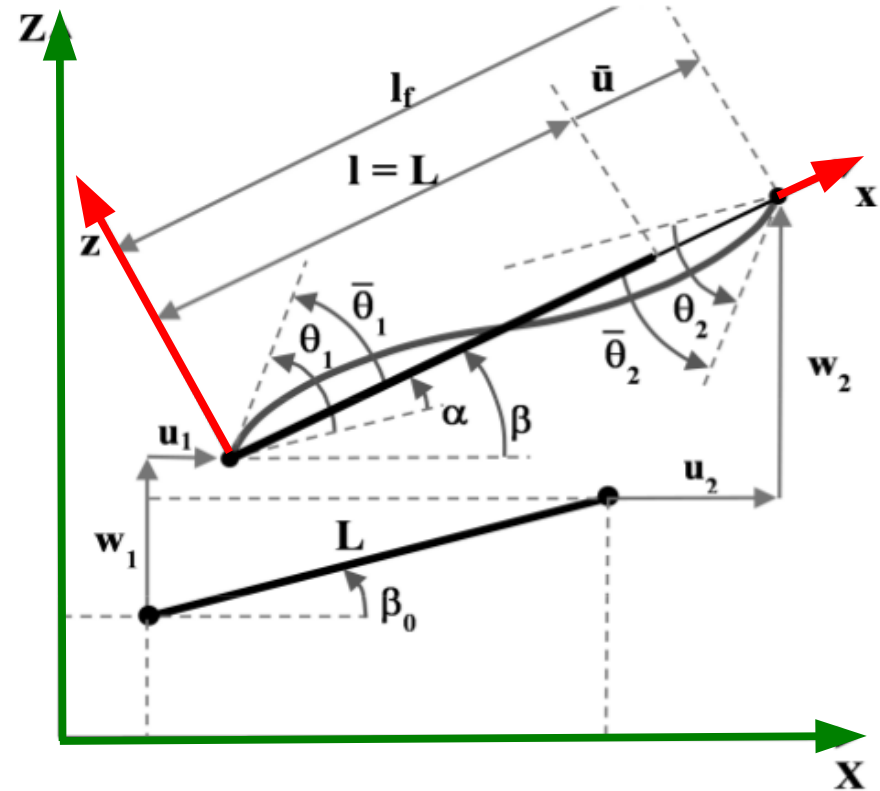
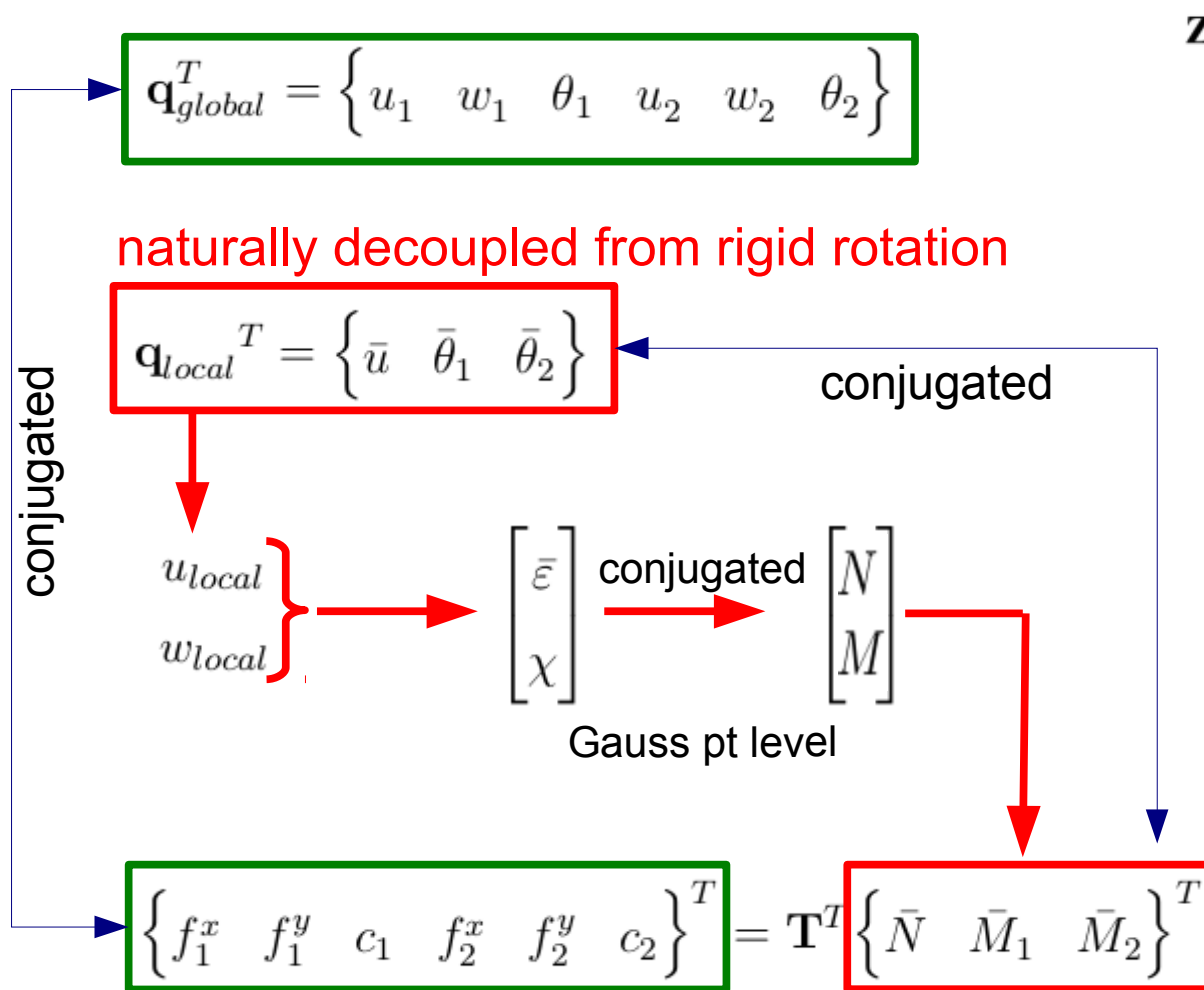
$$\left\{ \begin{matrix} f_1^x & f_1^y & c_1 & f_2^x & f_2^y & c_2 \end{matrix} \right\}^T = \mathbf{T}^T \left\{ \begin{matrix} \bar{N} & \bar{M}_1 & \bar{M}_2 \end{matrix} \right\}^T$$

$$c = \cos \beta = \frac{1}{l_f}(X_2 - X_1 + u_2 - u_1)$$

$$s = \sin \beta = \frac{1}{l_f} (Z_2 - Z_1 + w_2 - w_1)$$



Computation of internal forces - summary 11



$$\delta \mathbf{f}_{global,int} = \delta (\mathbf{T}^T \mathbf{f}_{local,int})$$

with $\mathbf{T} = \begin{bmatrix} 1 & 1/l_f & 1/l_f \\ -c & -s & 0 & c & s & 0 \\ s & -c & 1 & s & c & 0 \\ s & -c & 0 & s & c & 1 \end{bmatrix}$

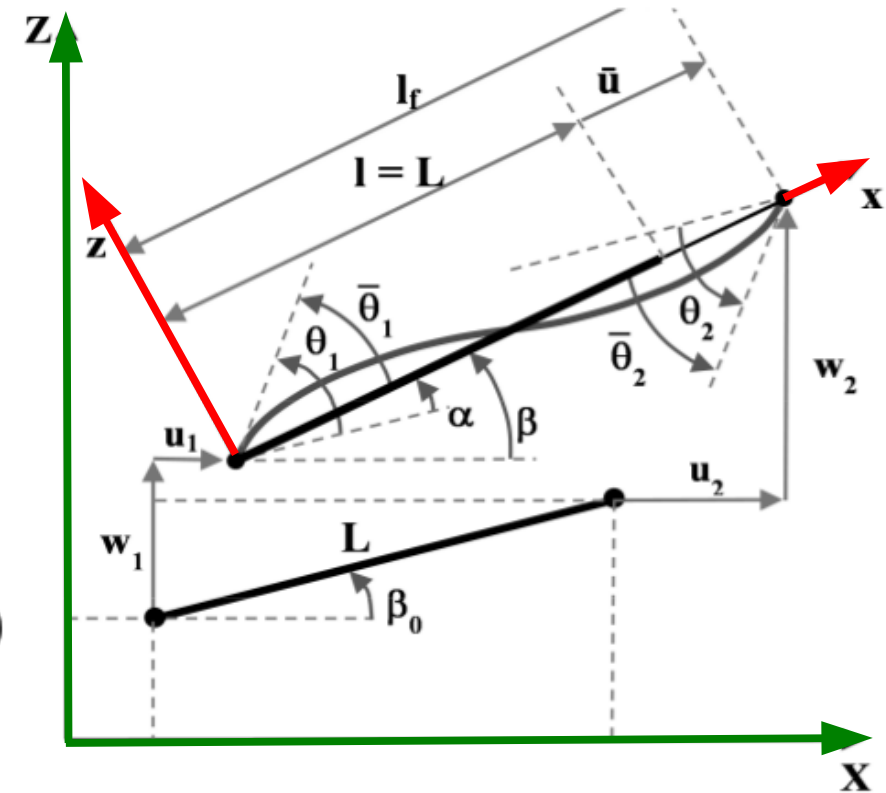
$$\delta \mathbf{f}_{global,int} = \mathbf{T}^T \delta \mathbf{f}_{local,int} + \bar{N} \delta \mathbf{t}_1 + \bar{M}_1 \delta \mathbf{t}_2 + \bar{M}_2 \delta \mathbf{t}_3$$

$$\mathbf{T}^T \delta \mathbf{f}_{local,int} = \mathbf{T}^T \mathbf{K}_{local} \delta \mathbf{q}_{local} = \mathbf{T}^T \mathbf{K}_{local} (\mathbf{T} \delta \mathbf{q}_{global})$$

$$\mathbf{K}_{local} = \int_l \mathbf{B}^T \mathbf{H} \mathbf{B} dl$$

sectional stiffness

$$\mathbf{H} = \frac{dE_{gen}}{d\Sigma_{gen}} \quad \text{with} \quad \mathbf{E}_{gen} = \begin{bmatrix} \bar{\epsilon} \\ \chi \end{bmatrix} \quad \Sigma_{gen} = \begin{bmatrix} N \\ M \end{bmatrix}$$



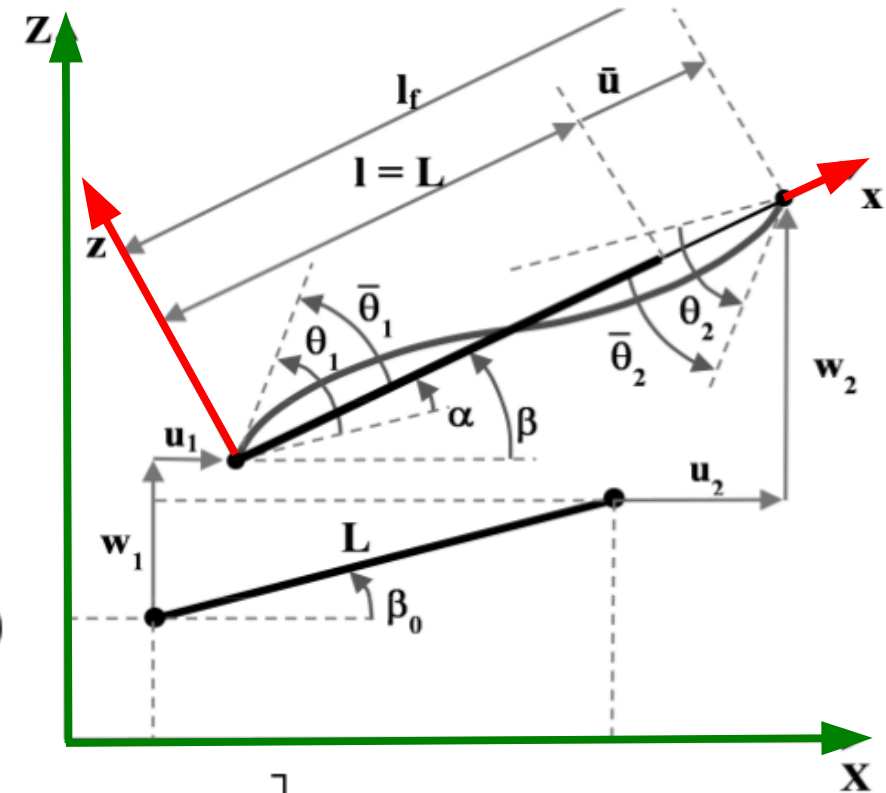


$$\delta \mathbf{f}_{global,int} = \delta (\mathbf{T}^T \mathbf{f}_{local,int})$$

with $\mathbf{T} = \begin{bmatrix} 1 & 1/l_f & 1/l_f \\ -c & -s & 0 & c & s & 0 \\ s & -c & 1 & s & c & 0 \\ s & -c & 0 & s & c & 1 \end{bmatrix}$

$$\delta \mathbf{f}_{global,int} = \mathbf{T}^T \delta \mathbf{f}_{local,int} + \bar{N} \delta \mathbf{t}_1 + \bar{M}_1 \delta \mathbf{t}_2 + \bar{M}_2 \delta \mathbf{t}_3$$

$$\mathbf{T}^T \delta \mathbf{f}_{local,int} = \mathbf{T}^T \mathbf{K}_{local} \delta \mathbf{q}_{local} = \mathbf{T}^T \mathbf{K}_{local} (\mathbf{T} \delta \mathbf{q}_{global})$$



$$\delta \mathbf{f}_{global,int} = \left[\mathbf{T}^T \mathbf{K}_{local} \mathbf{T} + \bar{N} \frac{\mathbf{z}\mathbf{z}^T}{l_f} + (\bar{M}_1 + \bar{M}_2) \frac{1}{l_f^2} (\mathbf{r}\mathbf{z}^T + \mathbf{z}\mathbf{r}^T) \right] \delta \mathbf{q}_{global}$$

$$\mathbf{K}_{global} = \mathbf{T}^T \mathbf{K}_{local} \mathbf{T} + \bar{N} \frac{\mathbf{z}\mathbf{z}^T}{l_f} + (\bar{M}_1 + \bar{M}_2) \frac{1}{l_f^2} (\mathbf{r}\mathbf{z}^T + \mathbf{z}\mathbf{r}^T)$$

Contribution of the
stress variation

Other terms: contribution of
the change in the geometry

$$c = \cos \beta = \frac{1}{l_f} (X_2 - X_1 + u_2 - u_1)$$

$$s = \sin \beta = \frac{1}{l_f} (Z_2 - Z_1 + w_2 - w_1)$$



J.-M. Battini, Corotational beam elements in instability problems, *Ph.D. Thesis*, Royal Institute of Technology, Department of Mechanics, Stockholm, Sweden, 2002.

C.E.M. Oliveira, A.G. Marchis, P.Z. Berke, R.A.M. Silveira, T.J. Massart, Computational analysis of a RC planar frame using corotational multilayered beam FE, correlated to experimental results, In Proceedings of XXXIV CILAMCE, 13 pages, 2013

